

STRUCTURAL ENGINEER MICHAEL HORTON ASSOCIATES, CONSULTING (MHAI)

151 MEADOW STREET, BRANFORD , CT 06405 Phone: (203) 481-8600

TOWN OF MONROE



SAINT JUDE SCHOOL **GYMNASIUM WALL REPAIR**

707 MONROE TURNPIKE, **MONROE CONNECTICUT 06468**

DATE: FEBRUARY 24, 2025

ASSOCIATES ARCHITECTURE + INTERIORS

271 Fairfield Avenue Bridgeport, Connecticut 06604

TEL (203) 377-1300 FAX (203) 378-3002

www.antinozzi.com

GRAPHICS LEGEND	ABBREVIATIONS
GRAPHICS LEGEND CONSTRUCTION PLAN SYMBOLS CONSTRUCTION PE CONSTRUCTION NOTE CONSTRUCTION CONSTRUCTION	ABBREVIATIONS AP, AF, ACUBITAC ECLINGTUE AF, ACUBITAC ECLINGTUE AF, ACUBITAC ECLINGTUE AF, ACUBITAC ECLINGTUE AF, ACUBITAC ECLINGTUE AF, ACUBITAC ELINGTUE AF, ACUBITAC ELINGTUE AF, AC
TILE CPT INDICATES FLOOR TRANSITION	H.O. HOLD OPEN (ELECTRO MAGNETIC) V.I.F. VERIFY IN FIELD H.P. HIGH POINT VERT. VERTICAL HR. HOUR W/ WITH HT. HEIGHT WASH.F. WASH FOUNTAIN HVAC HEATING, VENTILATION AND W.B. WHITEBOARD AIR CONDITIONING W.C. WATER CLOSET H.W. HOT WATER WD. WOOD I.D. INSIDE DIAMETER W/O WITHOUT

SITE LOCATION MAP



AN ASS ARC +	ΤΙΝ(50CI ΗΙΤΕ(ΙΝΤΕΓ	DZZI ATES CTURE RIORS		
COPYRIGHT 20 ANTINOZZI ASS 271 Fairfield AN Bridgeport, Co Tel: (203) 377- Fax: (203) 378- These docume specifically for other use of th without the ap	24 BY: OCIATES, P.C. /enue nnecticut 06604 1300 -3002 www ents have been this project. Re ese documents oproval of the A	v.antinozzi.com prepared production or is prohibited rchitect.		
CONSULTANT:				
	ESCRIPTION			
SAINT JUDE SCHOOL	gymnasium wall repair	707 Monroe Turnpike Monroe, connecticut 06468		
drawing title: DRAWING NOTES, A & GRAPH	DRAWING TITLE: DRAWING LIST, GENERAL NOTES, ABBREVIATIONS, & GRAPHICS LEGEND			
SCALE: AS NOTED DRAWING NO.	DRAWN BY: LRG	PAL		

24 FEBRUARY 2025 24034





A1 EXTERIOR WALL SECTION SCALE: 1" = 1'-0"



B2 WALL SECTION @ EXISTING ROOF JOIST



WEEP VENTS @ 24" O.C. -CONT. FLUID APPLIED AIR & -

COORDINATE GROOVE WITH -STONE SILL ANCHOR

CAST STONE SILL W/ INTEGRAL -

STAINLESS STEEL STONE SILL ANCHOR. COORDINATE W/ STONE

ANCHOR BOLT(S) SET IN EPOXY -

COMPACT 1" RIGID INSULATION -

P.T. SHIM -

SEALANT

SEALANT -----

GALVANIZED LOOSE LINTEL, SEE — STRUCTURAL DWGS.

WATER BARRIER

AIR SPACE ——

EXISTING BRICK.

TERMINATION BAR -

MORTAR NET —

P.T. SHIM ——

NOTE: REMOVE AND SALVAGE



A3 WALL SECTION @ EXISTING ROOF JOIST SCALE: 3" = 1'-0"

COPYRIGHT 202 ANTINOZZI ASSC 271 Fairfield Ave Bridgeport, Con Tel: (203) 377-13	4 BY: DCIATES, P.C. Enue necticut 06604 300	1
These documer specifically for t other use of the without the app	hts have been his project. Re se documents proval of the A	prepared production or is prohibited rchitect.
CERTIFICATION:		
CONSULTANT:		
REVISIONS:		
	SCRIPTION	
OL	٨R	88
CHO	LL REPA	RNPIKE CUT 064
JDE S	IM WA	NROE TU DNNECTI
VT JU	1NASIL	707 MO NROE, CC
SAIN	GY	MOM
drawing title: WALL SECT AND DETA	ions ils	
SCALE:		REVIEWED BY: PAL

	L NOTES	GENERAL NO
	AL	GENERAL
2022 CONNECTICUT STATE BUILDING COL	— IING CODE: 2022 CONNECT	GOVERNING
TOWN OF MONROE	LOADS: TOWN OF MO	DESIGN LOAD
M LIVE LOADS: GYMNASIUM:	MINIMUM LIVE LOADS:	М
DAD:	ROOF LOAD:	RC
ROOF SNOW LOAD CRITERIA: PG = 30 PSF CE = 0.9 IS = 1.0 CT = 1.0 PF = 22.6 PSF	ROOF SNOW I PG = 30 PSF CE = 0.9 IS = 1.0 CT = 1.0 PF = 22.6 PS	
WITH INCREASES FOR SNOW DRIFTING,	WITH INCREA	
MINIMUM ROOF LIVE LOAD $=$ 30 PSF	MINIMUM RO	
ROOF DEAD LOAD $= 15$ PSF	ROOF DEAD L	
$\begin{array}{llllllllllllllllllllllllllllllllllll$	WIND LOAD CRITERIA: ULTIMATE WI NOMINAL DE RISK CATEGO EXPOSURE CI	W
MINIMUM WIND LOAD ON PRIMARY STR	MINIMUM WI	
WIND LOADS ON SECONDARY ELEMENT	WIND LOADS	
COMPONENT AND CLADDING DESIGN W	COMPONENT	
ROOF ZONE 1: POSITIVE: 16.00 P NEGATI	ROOF ZONE 1	
ROOF ZONE 2: POSITIVE: 16.00 P NEGATI	ROOF ZONE 2	
ROOF ZONE 3: POSITIVE: 16.00 P NEGATI	ROOF ZONE 3	
WALL ZONE 4: POSITIVE: 16.00 P NEGATI	WALL ZONE 4	
WALL ZONE 5: POSITIVE: 16.00 P NEGATI	WALL ZONE 5	
ROOF OVERHANG ZONE 2: ROOF OVERHANG ZONE 3:	ROOF OVERHA ROOF OVERHA	
DESIGN WIND PRESSURE IS COMPUTED	DESIGN WINE	
CLOAD CRITERIA: AS PER SECTION 1 RISK CATEGORY = II SEISMIC IMPORTANCE FACTOR, IE = 1.0 SS = 0.2086, S1 = 0.055G SOIL SITE CLASS = D (ASSUMED) SPECTRAL RESPONSE COEFFICIENTS, SI SEISMIC DESIGN CATEGORY, B ORGINARY REINFORCED MASONRY SHEA DESIGN BASE SHEAR, V = 0.01W RESPONSE MODIFICATION FACTOR, R = ANALYSIS PROCEDURE USED: SIMPLIFIE	SEISMIC LOAD CRITERIA RISK CATEGO SEISMIC IMPO SS = 0.208G SOIL SITE CL/ SPECTRAL RE SEISMIC DES ORGINARY RE DESIGN BASE RESPONSE M ANALYSIS PR	SE
) ANY OF THE DETAILED INSTRUCTIONS SH CIFICATIONS, OR WITH EACH OTHER, THE S	SHOULD ANY OF THE DE THE SPECIFICATIONS, O	SH TH
RUCTURE IS DESIGNED TO BE SELF-SUPPOF CTOR'S SOLE RESPONSIBILITY TO DETERM LDING AND ITS COMPONENT PARTS DURIN G, TEMPORARY BRACING, GUYS OR TIEDOW CCTOR'S PROPERTY AFTER COMPLETION OF	THE STRUCTURE IS DES CONTRACTOR'S SOLE RE THE BUILDING AND ITS (HEETING, TEMPORARY B CONTRACTOR'S PROPER	TH CC TH HE CC

- 3.
- 4. PHASES OF CONSTRUCTION.
- BEING SUBMITTED TO THE ARCHITECT FOR APPROVAL.
- 6. WITH ANY WORK.
- 7.
- 8.
- 9.

CONTRACTOR'S DELEGATED DESIGN

- CONNECTICUT TO PERFORM THE DESIGN OF THE FOLLOWING:

DE, (2021 INTERNATIONAL BUILDING CODE). 100 PSF , UNBALANCES AND SLIDING PER SECTION 1608 (2021 IBC). 21 IBC) RUCTURE = 16 PSFS SHALL CONFORM WITH ASCE 7-16. VIND PRESSURES: VE: -46.00 PSF /E: -60.00 PSF IVE: -81.00 PSF IVE: -31.00 PSF VE: -38.00 PSF -41.00 PSF -56.00 PSF D BASED ON ULTIMATE WIND SPEED USING 10 SQUARE FOOT OF AREA. 1613 (2015 IBC) WITH: SDS = 0.222G, SD1 = 0.088GAR WALLS = 3.25 ED ANALYSIS IOWN ON THE PLANS CONFLICT WITH THE GENERAL STRUCTURAL NOTES, STRICTEST PROVISION SHALL GOVERN. TING AND STABLE AFTER THE BUILDING IS FULLY COMPLETED. IT IS THE /INE ERECTION PROCEDURE AND SEQUENCE AND TO INSURE THE SAFETY OF G ERECTION. THIS INCLUDES THE ADDITION OF WHATEVER SHORING, S NS WHICH MIGHT BE NECESSARY. SUCH MATERIAL SHALL REMAIN THE F THE PROJECT. THE CONTRACTOR SHALL PROVIDE SHORING CALCULATIONS AND SHORING DRAWINGS, INDICATING THE WORK TO BE PROVIDED, SIGNED AND SEALED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF CONNECTICUT. THE STRUCTURE UTILIZES SHEAR WALLS TO PROVIDE LATERAL STABILITY. THEREFORE, TEMPORARY BRACING, GUYS, ETC. MUST BE MAINTAINED UNTIL ALL MASONRY SHEAR WALLS HAVE BEEN ERECTED AND ATTACHED TO STEEL FRAMING. IT IS THE CONTRACTOR'S SOLE RESPONSIBILITY TO FOLLOW ALL APPLICABLE SAFETY CODES AND REGULATIONS DURING ALL

5. SHOP DRAWINGS ARE TO BE CHECKED BY THE CONTRACTOR AND SUBCONTRACTOR AND BEAR CHECKER'S INITIALS BEFORE

THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS, ELEVATIONS, ANGLES AND EXISTING CONDITIONS BEFORE PROCEEDING

ALL SECTIONS AND DETAILS SHALL BE CONSIDERED TYPICAL AND APPLY FOR THE SAME AND SIMILAR SITUATIONS THROUGHOUT THE BUILDING, UNLESS OTHERWISE SPECIFICALLY NOTED.

CONTRACTOR SHALL REVIEW ALL ARCHITECTURAL AND STRUCTURAL DRAWINGS PRIOR TO SUBMITTING THEIR BID FOR REFERENCE TO ALL NOTES ON ARCHITECTURAL DRAWINGS REFERRING TO ASEE STRUCTURAL DRAWINGS. IF THE SIZE OF ELEMENTS AND DETAILING OF MEMBERS IS NOT INDICATED, THE CONTRACTOR SHALL CONTACT THE ARCHITECT TO REQUEST THE MISSING INFORMATION IN PREPARATION OF THEIR BID. THESE REFERENCED ITEMS SHALL BE PART OF THE BASE BID.

IN CASES OF DISCREPANCIES BETWEEN CONTRACT DOCUMENTS AND SUBMITTED SHOP DRAWINGS, THE CONTRACT DOCUMENTS SHALL GOVERN INSTALLATION OF MATERIALS.

1. CONTRACTOR SHALL RETAIN THE SERVICES OF A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF

A. TEMPORARY SHORING OF EXISTING STRUCTURAL MEMBERS.

ALL CALCULATIONS SHALL BE SIGNED AND SEALED BY THE ENGINEER AND SUBMITTED FOR REVIEW.

STRUCTURAL STEEL

MATER	IALS:		1.	BAR SIZE
Struc Bolts Ancho Weldin	TURAL STEEL ASTM A 36 ASTM A325 DR BOLTS ASTM F1554, GRADE 36 NG ELECTRODE ASTM E 70		2. 3. 4. 5.	JOINT REIN #4 #5 #6 #7
1.	DESIGN, FABRICATION, AND ERECTION OF STRUCTURAL STEEL SHALL CONFORM TO AMERICAN INSTITUTE OF STEEL CONSTRUCTION SPECIFICATION, AISC 360-10. ALL REACTIONS SHOWN ON PLAN HAVE BEEN DEVELOPED USING ALLOWABLE STRESS DESIGN	22.	o. IF EPOX INCREA	#7 (Y COATED REIN (SED BY 50% PI
2.	WELDING SHALL CONFORM TO THE CODE FOR "ARC AND GAS WELDING IN BUILDING CONSTRUCTION" OF THE AMERICAN WELDING SOCIETY. ALL WELDING SHALL BE DONE BY A CERTIFIED WELDER.	23.	SUBMI SPECIF	T SHOP DRAWIN
3.	ALL LOOSE BEAM LINTELS SHALL HAVE 8" MINIMUM BEARING. SEE ARCHITECTURAL JAMB DETAILS FOR LENGTHS.	24.	JUINTS SUBMI	, and the req T Shop Drawi
4.	FOR MISCELLANEOUS STEEL NOT SPECIFICALLY DETAILED ON STRUCTURAL DRAWINGS, REFER TO ARCHITECTURAL DRAWINGS.	25.	COORD PROVID	INATE LOCATIO
5.	NO WELDING OR FINAL BOLTING SHALL BE DONE UNTIL AS MUCH OF THE STRUCTURE THAT WILL BE STIFFENED THEREBY HAS BEEN PROPERLY ALIGNED.		MAIN A FROM (ND INTERSECT CORNERS.
6. 7	SEQUENCE OF PLACING WELDS SHALL BE SUCH AS TO AVOID DISTORTION OF MEMBERS.	26.	all re Utilize	INFORCING, TH HILTI HIT-HY2
1.	OWNER. SUBSTITUTED MEMBERS MUST BE OF THE SAME NOMINAL DEPTH AS THE MEMBER ORIGINALLY INDICATED AND HAVE A WEIGHT GREATER THAN THAT INDICATED. BEAM FLANGES MUST NOT INFRINGE ON ADJACENT ARCHITECTURAL ELEMENTS.	27.	all re Utilize	INFORCING, TH E HILTI HIT-HY20
8.	BEAMS BEARING ON MASONRY SHALL HAVE ANGLE WALL ANCHORS WELDED TO THE BEAM, AS DETAILED IN THE A.I.S.C. MANUAL OF STEEL CONSTRUCTION.	CEME	NTITIOUS R	OOF DECK
9.	ALL STEEL MEMBERS EXPOSED TO WEATHER OR IN CONTACT WITH PRESSURE TREATED LUMBER OR WOOD PRODUCTS IN THE COMPLETED CONSTRUCTION SHALL BE HOT-DIPPED GALVANIZED IN ACCORDANCE WITH ASTM A123.	1.	a. b.	
10. 11.	PROVIDE BITUMASTIC PROTECTION COATING FOR ALL STRUCTURAL STEEL BELOW GRADE. EXISTING STEEL SURFACES TO RECEIVE FIELD WELDS SHALL BE THOROUGHLY CLEANED UNTIL FREE FROM PAINT, RUST,		C.	ACOUSTIC I 3" THICKNE
12.	GREASE, ETC. STEEL CONTRACTOR SHALL PROVIDE SKETCHES FOR ALL CORRECTIVE FIELD WORK WHICH SHALL BE SUBMITTED TO THE ENGINEER OF RECORD FOR APPROVAL. ALL SKETCHES SHALL BE SIGNED AND SEALED BY THE STEEL FABRICATORS	2.	PROVID PANEL INSTAL	DE DRAWINGS II Layout, attac Lation instru
13.	CONNECTIONS ENGINEER. ALL THREADED RODS OR BOLTS INDICATED TO BE DRILLED AND EPOXIED SHALL UTILIZE HILTI HIT-HY200 ADHESIVE OR	3.	INSTAL	LER QUALIFICA
CONCF	APPROVED EQUAL.		a. b.	UTILIZE AN LETTER OF AND IS CER
MATER	IALS:	4.	REGUL	ATORY REQUIRE
	HOLLOW LOAD BEARING UNITS: ASTM C 90 (NET AREA COMPRESSIVE STRENGTH OF CMU UNIT = 2000PSI)		a.	INTERNATIO
	MORTAR: (TYPE S) ASTM C 270 (COMPRESSIVE STRENGTH OF MASONRY ASSEMBLY, F'M = 2000 PSI) GROUT FOR REINFORCED MASONRY, ASTM C 476	5.	WARRA a	
	(COMPRESSIVE STRENGTH AT 28 DAYS = 2500 PSI) GROUT FOR REINFORCED MASONRY: ASTM C 476		a. b.	MANUFACT
	SOLID LOAD BEARING UNITS: (GRADE N-I) ASTM C 145 CONCRETE BRICK: (GRADE N-I) ASTM C 55	6.	TOTAL	PANEL THICKN
1.	WALLS INDICATED ON STRUCTURAL DRAWINGS ARE FOR REFERENCE ONLY. SEE ARCHITECTURAL DRAWINGS FOR LOCATION, THICKNESS AND COMPOSITION OF MASONRY WALLS.			<u>]</u> 3
2.	ALL MASONRY WALLS SHALL CONTAIN THE FOLLOWING REINFORCING:	7.	DECK E ASPEN	BOARD SHALL B WOOD FIBERS
	1-#5 VERTICAL BAR AT 24" ON CENTER. 2-#6 VERTICAL BARS AT BOTH SIDES OF DOOR, WINDOW AND MECHANICAL OPENINGS.	8.	ACCES	SORIES:
	2-#5 HORIZONTAL BAR MINIMUM ABOVE AND BELOW ALL WINDOW AND MECHANICAL OPENINGS AND ABOVE ALL		A.	FASTENERS 1. [
	REQUIRED IN ACCORDANCE WITH LINTEL SCHEDULE ON ARCHITECTURAL DRAWINGS.		В.	WASHERS: 1. 2
	2-#3 HONZONTAL AT TOP OF ALL WALLS, AND AT BOND BEAMS CONNECTED TO FLOORS AND ROOFS, UNLESS OTHERWISE INDICATED.		C.	ADHESIVE: 1.
	2-#6 VERTICAL BARS AT ENDS OF ALL WALLS, AND EACH SIDE OF CONTROL JOINTS.		D.	GROUT: 1.
3.	SHALL BE #9 GAGE WIRE. PROVIDE VERTICAL DOWELS FROM CONCRETE WALLS INTO ALL CMU WALLS. SIZE AND SPACING OF THE DOWELS SHALL MATCH THE VERTICAL REINFORCING AS SPECIFIED IN THESE GENERAL NOTES. JUNI ESS OTHERWISE NOTED ON THE		E.	F STEEL CHA 1.
	DRAWINGS. DOWEL LENGTHS SHALL BE THE REQUIRED CONCRETE DEVELOPMENT LENGTH PLUS THE REQUIRED BAR LAP SPLICE LENGTH FOR MASONRY AS SPECIFIED IN THESE GENERAL NOTES.	9.		LATION: COMP
4.	ALL VERTICAL WALL REINFORCING SHALL BE CONTINUOUS FOR THE FULL HEIGHT OF MASONRY WALLS, INCLUDING THROUGH CONTINUOUS MASONRY BOND BEAMS UNLESS OTHERWISE INDICATED.		A.	PLACE PAN
5.	ALL GROUTING OF MASONRY WALLS SHALL BE ASSUMED TO BE COMPLETED BY LOW LIFT GROUTING METHODS. IF THE CONTRACTOR PROPOSES TO UTILIZE HIGH LIFT GROUTING METHODS THEY SHALL SUBMIT THEIR PROPOSED HIGH LIFT GROUTING PROCEDURE FOR REVIEW PRIOR TO STARTING ANY GROUTING ON THE PROJECT SITE.		В.	STAGGER E STRIPS OR
6.	REINFORCING ABOVE WINDOWS, DOORS AND MECHANICAL OPENINGS IN THE EXTERIOR WALLS SHALL BE IN A BOND BEAM COURSE ABOVE THE STEEL LINTELS PROVIDED AT THESE OPENINGS. BOND BEAMS SHALL EXTEND 2'-0" BEYOND THE		C.	Support P Including
7.	OPENING. BOND BEAM LINTELS SHALL BE CONSTRUCTED USING SOLID BOTTOM "U" BLOCK MASONRY UNITS.		D.	PANELS RE SCREWED /
8.	CELLS CONTAINING REINFORCING BARS AND ALL CELLS BELOW GRADE SHALL BE GROUTED SOLID. ALL OTHER CELLS SHALL		E.	PANEL END MINIMUM (
0	REINFORCING.		F.	CUT PANEL
9.	ALL BOLTS OF ANCHORS SHALL BE SOLIDET EMBEDDED IN MORTAN OF GROUT. IF BOND BEAM IS NOT LOCATED AT BOLT OF ANCHOR ELEVATION, PROVIDE LATH AND FILL CELL LOCALLY TO PROVIDE SUBSTRATE FOR BOLT OR ANCHOR. GROUT CELL ABOVE ALL MASONRY ANCHORS.		G.	APPLY ADH
10.	ALL COLUMNS WITHIN WALLS SHALL BE SOLIDLY EMBEDDED IN GROUT.		H.	
11.	GROUT SOLID MASONRY FOR FULL HEIGHT OF WALL BELOW EACH LOOSE LINTEL AND PROVIDE #4 VERTICAL IN NEW OR EXISTING CORE.		I.	INSTALL SC
12.	GROUT SOLID MASONRY FOR TWO COURSES BELOW EACH BEAM BEARING EXCEPT AS NOTED.		J.	
13. 1⊿	USE 1 COURSE (8") OF SOLID MASONRY OR GROUTED SOLID MASONRY BELOW EACH STEEL JOIST BEARING EXCEPT AS NOTED.			OVERLAY S OVERLAY W
ı .	FRAMING. GROUT CELL ABOVE ANCHOR.	9.	PROTE(CTION:
15.	HOLLOW UNITS SHALL BE LAID WITH FULL MORTAR COVERAGE ON HORIZONTAL AND VERTICAL FACE SHELLS, EXCEPT THAT WEBS SHALL ALSO BE BEDDED IN ALL COURSES OF BEARING AND SHEAR WALLS, PIERS, COLUMNS AND PILASTERS, AND IN THE STARTING COURSE ON FOOTINGS AND SOLID FOUNDATION WALLS, AND WHERE ADJACENT TO CELLS OR CAVITIES WHICH ARE TO BE REINFORCED AND/OR FILLED WITH GROUT.		A. B.	
16.	MORTAR PROTRUSIONS EXTENDING INTO CELLS OR CAVITIES TO BE REINFORCED AND/OR GROUTED SHALL BE REMOVED.		U.	EXCESSIVE
17.	ALL MASONRY WALLS SHALL BE BRACED AT THE TOP WHERE MASONRY ENDS AT THE UNDERSIDE OF FLOOR OR ROOF CONSTRUCTION. REFER TO TYPICAL DETAILS.			
18.	SOLID UNITS SHALL BE LAID WITH FULL HEAD AND BED JOINTS.			
19.	COLLAR (VERTICAL LONGITUDINAL) JOINTS BETWEEN THE FACING AND BACKING WYTHES IN WALLS SHALL BE COMPLETELY FILLED WITH MORTAR OR GROUT AND WORKED IN WITH A TROWEL.			

21.

20. ALL INTERSECTING LOAD BEARING WALLS SHALL BE TIED TOGETHER IN MASONRY BOND UNLESS NOTED OTHERWISE.

MINIMUM DEVEL	OPMENT LENGTH AND SPLICE LENGTH OF MASONRY	REINFORCING SHALL BE AS FOLLOWS:		ASSOCIATES
1. <u>BAR S</u> 2. JOINT	<u>DEVELOPMENT LENGTH</u> REINFORCING 9"	<u>SPLICE LENGTH</u> 12"		
3. #4 4. #5 5. #6 6 #7	18" 25" 27" 32"	24" 30" 36" 42"		+ INTERIORS
IF EPOXY COATE	D REINFORCING IS SPECIFIED IN THE MASONRY SPECI 0% PER THE ACI 530 MASONRY CODE.	FICATIONS, THEN ALL SPLICE LENGTHS SHALL BE		COPYRIGHT 2023 BY:
SUBMIT SHOP D SPECIFICATIONS JOINTS, AND THI	RAWINGS INDICATING THE PLACEMENT OF ALL REINFO FOR SUBMITTAL REQUIREMENTS. SHOP DRAWINGS S E REQUIRED LAP SPLICES FOR ALL REINFORCING.	DRCING REQUIRED IN MASONRY WALLS. REFER TO HALL INDICATE THE LOCATION OF ALL CONTROL		ANTINOZZI ASSOCIATES, P.C. 271 Fairfield Avenue Bridgeport, Connecticut 06604 Tel: (203) 377-1300
SUBMIT SHOP D COORDINATE LO	RAWINGS INDICATING THE PLACEMENT OF TOP OF WA CATIONS WITH ARCHITECTURAL DRAWINGS.	LL PARTITION ANCHORS AT ALL INTERIOR CMU WA	LLS.	Fax: (203) 378-3002 www.antinozzi.com
PROVIDE MASON Main and inter From Corners	IRY CONTROL JOINTS AT A MAXIMUM SPACING OF 30 I RSECTING WALLS, AT CHANGES IN WALL HEIGHT, CHAI	FEET ON CENTER. PROVIDE CONTROL JOINTS BETW NGES IN WALL THICKNESS AND NO GREATER THAN	/EEN 4'-0"	specifically for this project. Reproduction or other use of these documents is prohibited without the approval of the Architect.
ALL REINFORCIN UTILIZE HILTI HIT	G, THREADED RODS OR BOLTS INDICATED TO BE DRILI -HY270 ADHESIVE OR APPROVED EQUAL.	LED AND EPOXIED INTO HOLLOW CMU OR BRICK SH	IALL	CERTIFICATION:
all reinforcin Utilize hilti hit	G, THREADED RODS OR BOLTS INDICATED TO BE DRILI -HY200 ADHESIVE OR APPROVED EQUAL.	ED AND EPOXIED INTO SOLID GROUTED CMU SHAL	L	
TITIOUS ROOF DEC	<u>K</u> MENTS [,] PROVIDE ROOF DECK ASSEMBLY DESIGNED A	ND TESTED ACCORDING TO THE FOLLOWING:		
a. UNDE b. STRU FABRI c. ACOU	RWRITERS LABORATORIES UL 580 CTURAL PERFORMANCE REQUIREMENTS: PROVIDE A R CATED, AND INSTALLED TO PROVIDE DEFLECTION OF STIC PERFORMANCE: NRC VALUE 0.60 MINIMUM GENE	00F DECK SYSTEM THAT HAS BEEN MANUFACTURI TL L/240 AND LL L360 RATED FROM UL CLASSIFIED FACE MATERIAL OF 2	ЕD, " ТО	CONSULTANT:
3" TH Provide Drawii Panel Layout, Installation in	ICKNESS. NGS INDICATING LOCATIONS AND SPACING OF STRUC ATTACHMENT DETAILS AND TERMINATION DETAILS. SI ISTRUCTIONS.	TURAL SUPPORTS AND PENETRATIONS INCLUDING JBMIT MANUFACTURER'S PRODUCT DATA AND		Michael Horton
				Associates Inc.
a. UTILIZ b. LETTE AND I	E AN INSTALLER HAVING DOCUMENTED EXPERIENCE (R OF CERTIFICATION FOR THE MANUFACTURER STATIN S CERTIFIED TO INSTALL SYSTEM.	ON PROJECTS OF A SIMILAR SIZE AND COMPLEXITY	ENCE	780 East Main Street Branford, Connecticut 06405 203-481-8600 mha-eng.com
REGULATORY RE a. INTER	QUIREMENTS AND APPROVALS SHALL COMPLY WITH I	REPORT ESR-1112.		REVISIONS
WARRANTY a. PROJI b. MANI	ECT WARRANTY: REFER TO CONDITIONS OF THE CONTF JFACTURER'S WARRANTY: SUBMIT MANUFACTURER'S	ACT FOR PROJECT WARRANTY PROVISIONS. STANDARD 15 YEAR THERMAL PERFORMANCF		
WARF	ICKNESS AND R-VALUE			
	TOTAL THICKNESSR-VALUE3"6.43			
DECK BOARD SH ASPEN WOOD FII SUBMITTED FOR ACCESSORIES:	IALL BE CEMENTITIOUS WOOD FIBER BOARD 3 INCH/NI BERS BONDED WITH INORGANIC HYDRAULIC CEMENT REVIEW.	RC 0.80 THICK TECTUM CONSISTING OF FSC CERTIF UNLESS NOTED OTHERWISE OR APPROVED EQUAL I	FIED S	
A. FASTE 1.	ENERS: DEKFAST 14 GAGE STEEL. LENGTH TO PENETRAT	E STRUCTURAL MEMBER MINIMUM OF 1-1/2 INCH.		
B. WASH 1.	IERS: 2" WASHERS			
C. ADHE 1.	SIVE: SFA-66			
D. GROU 1.	T: TECTUM GROUT (FOR USE WITH TECTUM TILE). G POTABLE WATER.	YPSUM CEMENT GROUT, READY FOR MIXING WITH		
E. STEEL 1.	. CHANNEL 16 GAUGE STEEL CHANNEL (FOR USE WITH TECT ALLOW FOR SPANS UP TO SIX FEET.	UM LONG SPAN). APPLIED TO STANDARD PLANK T()	
INSTALLATION: (PROCEDURES.	COMPLY WITH MANUFACTURER'S PUBLISHED INSTRUC	TIONS AND RECOMMENDED INSTALLATION		J Dair
A. PLACI B. STAGI	E PANEL ON JOISTS WITH SQUARE CUT ENDS BUTTED GER END JOINTS. SEAL JOINTS LARGER THAN 1/4 INCH	TIGHTLY TOGETHER. I WITH ADHESIVE AND LARGER JOINTS WITH FOAM		Ref Ref
C. SUPP	S OR EXPANDING FOAM. ORT PANELS WITH BENT PLATES (STEEL OR OTHER SU	PPORT MATERIAL) AT ROOF TRANSITIONS.		Scl Vall Uurnp 0646
D. PANE SCRE	LS REQUIRE A MINIMUM 1-INCH BEARING ON STRUCTU WED AT TRANSITIONS.	N, AND FANEL DIRECTION CHANGE CONDITIONS.		C C C C
E. PANE MININ	L ENDS ARE REQUIRED TO TERMINATE OVER STRUCTU /IUM OF 1 INCH BEARING ON STRUCTURAL MEMBERS.	RAL MEMBERS OR SUPPORTS WITH A		and Moi Moi onro
F. CUT F RECO	PANELS NEATLY TO ABUT TO PARAPETS AROUND OPEN MMENDED SAW AND TECHNIQUES FOR FIELD CUTTING	INGS AND PENETRATIONS. USE MANUFACTURER		$\mathbf{J}_{\mathbf{M}}$
G. APPL` MANU	Y ADHESIVE TO SUPPORT MEMBERS AND ON TOP OF P JFACTURER'S RECOMMENDATIONS.	LANK TONGUE IN ACCORDANCE WITH		jyn it
H. USE N PANE	/ANUFACTURER'S RECOMMENDED SLIDE HAMMER OF L JOINTS. HOLD PANELS IN POSITION UNTIL SCREWS A	OTHER TOOLS TO ASSURE A TIGHT JOINT AT PANE ARE INSTALLED.	L-T0-	\sim
I. INSTA MANU	LL SCREWS AT EACH STRUCTURAL SUPPORT IN CONF JFACTURER'S RECOMMENDED SPACING AND QUANTIT	ORMANCE WITH APPROVED SHOP DRAWINGS AND Y.		
J. FIELD OVERI OVERI	INSTALLED OVERLAY: IF INDICATED ON SHOP DRAWIN LAY STAGGERING PANEL JOINTS IN BOTH DIRECTIONS LAY WITH ADHESIVE AND 1-INCH STAPLES AS INDICAT	IGS, INSTALL CONTINUOUS 7/16-INCH OSB FOR A MINIMUM JOINT COVERAGE OF 1'-0". INSTAI ED ON APPROVED SHOP DRAWINGS.	L	
PROTECTION:				
A. PROTI B. PROTI	ECT INSTALLED WORK FROM DAMAGE DUE TO WEATH	ER RELATED MOISTURE. QUENT CONSTRUCTION ACTIVITY.		DRAWING TITLE: GENFRAL NOTES
C. PROV EXCES	IDE TEMPORARY PROTECTION AS NECESSARY TO PROT SSIVE MOISTURE PRIOR TO INSTALLATION OF ROOFING	ECT INSTALLED MATERIAL FROM EXPOSURE TO MATERIAL.		
				SCALE DRAWN BY: REVIEWED BY: 1/8" = 1'-0" LVP PIS
				DRAWING NO.
				S-000

DTES	

JOB NUMBER: 24-88

02/25/2024





ROOF CONSTRUCTION NOTES:

1.

(E) EXISTING ROOF CONSTRUCTION: 3" INSULATING ROOF PLANK. (N)

NEW ROOF CONSTRUCTION: 3" TECTUM I ROOF DECK TILE. LENGTH AND WIDTH OF TILE MUST BE FIELD VERIFIED PRIOR TO CONSTRUCTION . REFER TO ARCHTECTRAL DRAWING FOR ADDITIONAL DECK PANEL REQUIREMENTS. FASTEN DECKING TO SUBSTRATE WITH #10 SCREW FASTENERS AT 6" O.C.

2. CONTRACTOR TO FIELD VERIFY ALL EXISTING FRAMING SIZES, SPACING AND LOCATIONS PRIOR TO SUBMITTING SHOP DRAWINGS.

3. EXISTING STRUCTURE IS BASED ON LIMITED FIELD EXPLORATION AND EXISTING DRAWINGS PREPARED BY FLETCHER-THOMPSON, INC.

FOUNDATION NOTES:

- 1. TOP OF CONCRETE SLAB ELEVATION = REFER TO PLAN
- CONTRACTOR TO FIELD VERIFY ALL EXISTING FRAMING SIZES, SPACING AND LOCATIONS PRIOR TO SUBMITTING SHOP DRAWINGS. 2.

EXISTING STRUCTURE IS BASED ON LIMITED FIELD EXPLORATION AND EXISTING 3. DRAWINGS PREPARED BY FLETCHER-THOMPSON, INC.

	ANTINOZZA ASSOCIATES ARCHITECTURE + INTERIORSCOPYRIGHT 2023 BY: ANTINOZZI ASSOCIATES, P.C. 271 Fairfield Avenue Bridgeport, Connecticut 06604 Tel: (203) 377-1300 Fax: (203) 378-3002 MWW.antinozzi.comMesse documents have been prepared specifically for this project. Reproduction or other use of these documents is prohibited without the approval of the Architect.CERTIFICATION:
T EXISTING SLAB AT MAIN LEVEL	CONSULTANT: CONSULTANT: Michael Horton Associates Inc. Consulting Structural Energy Branford, Connecticut 06405 203-481-8600 mha-eng.com REVISIONS NO. DATE DESCRIPTION
UNEXCAVATED	St. Jude School Gymnasium Wall Repair 707 Monroe Turnpike Monroe, CT 06468
ATIONS	DRAWING TITLE: FOUNDATION AND ROOF PLAN



MASONRY LINTEL SCHEDULE_					
MARK	CMU	MU OPENING WIDTH LINTEL			
	10" & 12" BLOCK	GREAT THAN 4'-0" AND TO 12'-0"	(2)-#6 CONT. TOP AND BOTTOM	1 ¹ - 4 ¹¹	
	10" & 12" BLOCK	UP TO 4-0"	(2)-#5 CONT.	o _j	

SCHEDULE NOTE:

PROVIDE LINTELS WHERE NEEDED. NOT SHOWN ON THE DRAWINGS

ALL EXTERIOR STEEL SHALL BE HOT DIPPED GALVANIZED COORDINATE ALL OPENINGS WITH ARCHT DRAWINGS AND FIELD VERIFY TO MATCH EXISTING UNLESS NOTED

OTHERWISE GROUT ALL JAMBS SOLID PER TYPICAL CMU WALL REINF. DETAILS. 4.







_OCATION		ANTIN ASSOC	OZZI IATES
	ROOF 32' - 5 5/8"	ARCHITE + INTE	ECTURE ERIORS
L6X6X3/8	<u>Upper level</u> 21' - 4"	COPYRIGHT 2023 BY: ANTINOZZI ASSOCIATES, 271 Fairfield Avenue Bridgeport, Connecticut 066 Tel: (203) 377-1300 Fax: (203) 378-3002 w These documents have bee specifically for this project. other use of these documen without the approval of the A CERTIFICATION:	P.C. 04 /ww.antinozzi.com n prepared Reproduction or ts is prohibited Architect.
	<u>Main Level</u> 10' - 0"	CONSULTANT:	AT
	Lower Level 0"	NO. DATE DES	AI Horton S Inc. Engineers 06405 g.com
		Image: Constraint of the second sec	
PQF 5/8"		ol spair	
er level 21' - 4"		Jude Scho nasium Wall Re	07 Monroe Turnpike Monroe, CT 06468
n Level 10' - 0"		St. J Gym	7
r Level 0"		DRAWING TITLE: EXTERIOR BUILDIN ELEVATIONS	G
		SCALE DRAWN E 3/16" = 1'-0" AC/L DRAWING NO.	BY: REVIEWED BY: VP PJS
		S-2	JOB NUMBER:
		02/25/2024	24-88